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General Notes

GN. GENERAL

GN.1	THE STRUCTURAL DRAWINGS AND SPECIFICATIONS ARE A PORTION OF THE CONSTRUCTION DOCUMENTS. THE CONTRACTOR AND SUBCONTRACTORS SHALL REFERENCE AND COORDINATE WITH ALL OTHER DISCIPLINES' DRAWINGS. ANY DISCREPANCIES OR OMISSIONS SHALL BE REPORTED TO THE STRUCTURAL ENGINEER AND ARCHITECT.
GN.2	DESIGN CRITERIA:
A.	CODES AND SPECIFICATIONS:
1.	GENERAL BUILDING CODE: NORTH CAROLINA STATE BUILDING CODE, 2018 EDITION. (BASED ON INTERNATIONAL BUILDING CODE, 2015 EDITION).
2.	DESIGN LOAD CRITERIA: MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, AMERICAN SOCIETY OF CIVIL ENGINEERS, ASCE 7.
3.	CONCRETE: BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE, AMERICAN CONCRETE INSTITUTE, ACI 318.
4.	TIMBER: NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION, AMERICAN FOREST & PAPER ASSOCIATION/AMERICAN WOOD COUNCIL.
5.	COLD-FORMED STEEL NORTH AMERICAN SPECIFICATION FOR DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS, AMERICAN IRON AND STEEL INSTITUTE.
6.	MASONRY: BUILDING CODE REQUIREMENTS AND SPECIFICATION FOR MASONRY STRUCTURES, TMS 402/602.
B.	DESIGN LOADS (PSF):
	RISK CATEGORY-----II
1.	DEAD LOADS: ANY CHANGES IN CONSTRUCTION MATERIALS FROM THOSE SHOWN ON THE ARCHITECTURAL OR STRUCTURAL DRAWINGS SHALL BE REPORTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER FOR VERIFICATION OF LOAD-CARRYING CAPACITY OF THE STRUCTURE.
2.	LIVE LOADS: ROOF (REDUCIBLE)-----20
	LIVE LOAD REDUCTIONS HAVE BEEN APPLIED IN ACCORDANCE WITH THE BUILDING CODE, UNLESS NOTED.
3.	SNOW LOAD: GROUND SNOW LOAD (Pg)-----10.0 FLAT-ROOF SNOW LOAD (Pf)-----7.0 SNOW EXPOSURE FACTOR (Ce)-----1.0 SNOW LOAD IMPORTANCE FACTOR (Is)-----1.0 THERMAL FACTOR (Ct)-----1.0
4.	WIND LOADS: ULTIMATE DESIGN WIND SPEED, Vult-----115 MPH (3 - SECOND GUST) NOMINAL DESIGN WIND SPEED, Vasd-----89 MPH (3 - SECOND GUST) WIND EXPOSURE CATEGORY-----B INTERNAL PRESSURE COEFFICIENT-----0.18
	WALL COMPONENT AND CLADDING WIND PRESSURE-SEE DRAWINGS
5.	SEISMIC LOADS: SEISMIC IMPORTANCE FACTOR (Ie)-----1.0
	MAPPED SPECTRAL RESPONSE ACCELERATIONS: Ss-----0.221 S1-----0.098 ASSUMED SITE CLASS-----D DESIGN SPECTRAL RESPONSE ACCELERATION PARAMETERS: SDs-----0.235 SD1-----0.156 SEISMIC DESIGN CATEGORY-----C BASIC SEISMIC-FORCE-RESISTING SYSTEM: LIGHT-FRAME (WOOD) WALLS SHEATHED WITH WOOD STRUCTURAL PANELS RATED FOR SHEAR RESISTANCE SPECTRAL RESPONSE COEFFICIENT (Cs)-----0.036 RESPONSE MODIFICATION FACTOR (R)-----6.5 ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE METHOD

GN.3 CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND SITE CONDITIONS PRIOR TO FABRICATION/CONSTRUCTION. NOTIFY STRUCTURAL ENGINEER AND ARCHITECT OF ANY DISCREPANCIES PRIOR TO FABRICATION/CONSTRUCTION.

GN.4	SPECIAL INSPECTIONS/STRUCTURAL ENGINEER'S SITE VISITS:
A.	SPECIAL INSPECTIONS ARE REQUIRED FOR THIS PROJECT. REFER TO DRAWINGS.
B.	SITE VISITS BY THE STRUCTURAL ENGINEER'S OFFICE DO NOT REPLACE INSPECTIONS AND TESTING BY THE TESTING AGENCY OR SPECIAL INSPECTOR.

GN.5 SUBMITTALS:

A. REVIEW OF SHOP DRAWINGS AND OTHER SUBMITTALS BY THE STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY TO REVIEW AND CHECK SHOP DRAWINGS BEFORE SUBMITTING TO THE STRUCTURAL ENGINEER. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS. ALL SHOP DRAWINGS MUST BE REVIEWED AND "APPROVED" BY THE CONTRACTOR PRIOR TO SUBMITTAL.

B.	ELECTRONIC SHOP DRAWING SUBMITTALS: SUBMIT ALL ELECTRONIC SHOP DRAWINGS IN .PDF FORMAT. REVIEWED SHOP DRAWINGS WILL BE RETURNED IN .PDF FORMAT. ALL PRINTS REQUIRED BY THE CONTRACTOR ARE THE RESPONSIBILITY OF THE CONTRACTOR AND SHALL BE MADE AFTER APPROVED SHOP DRAWINGS ARE RETURNED.
C.	RESUBMITTED SHOP DRAWINGS: RESUBMITTED SHOP DRAWINGS SHALL HAVE ALL CHANGES SINCE THE PREVIOUS SUBMISSION IDENTIFIED BY CLOUDING OR OTHER CLEAR COMMUNICATION. RE-REVIEWED SHOP DRAWINGS WILL ONLY BE REVIEWED FOR IDENTIFIED CHANGES.
D.	SHOP DRAWINGS: THE CONTRACTOR SHALL SUBMIT FOR STRUCTURAL ENGINEER REVIEW SHOP DRAWINGS FOR THE FOLLOWING ITEMS. ITEMS MARKED (*) SHALL HAVE SHOP DRAWINGS SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE IN WHICH THE PROJECT IS LOCATED.
1.	CONCRETE MIX DESIGNS
2.	CONCRETE REINFORCING
3.	COLD-FORMED METAL FRAMING
4.	SHOP FABRICATED WOOD TRUSSES (*)
5.	STRUCTURAL STEEL
6.	MASONRY MORTAR MIX DESIGNS
7.	MASONRY GROUT MIX DESIGNS
8.	MASONRY REINFORCING
E.	DESIGN CALCULATIONS: THE CONTRACTOR SHALL SUBMIT FOR STRUCTURAL ENGINEER'S RECORD, DESIGN CALCULATIONS SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE IN WHICH THE PROJECT IS LOCATED FOR THE FOLLOWING ITEMS.
1.	SHOP FABRICATED WOOD TRUSSES
2.	COLD-FORMED METAL FRAMING

GN.6	ALL DETAILS SHOWN ARE TYPICAL. SIMILAR DETAILS APPLY TO SIMILAR CONDITIONS, UNLESS NOTED.
GN.7	THE CONTRACTOR IS RESPONSIBLE FOR MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.
GN.8	CONSTRUCTION MATERIALS SHALL BE SPREAD OUT IF PLACED ON FRAMED ROOFS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT LOADS DO NOT EXCEED THE DESIGN LIVE LOAD.

FD. FOUNDATION

FD.1	A GEOTECHNICAL ENGINEER, EMPLOYED BY THE CONTRACTOR, SHALL PROVIDE COMPACTED FILL REQUIREMENTS FOR THE BUILDING PAD AND REVIEW THE FOUNDATION BEARING SURFACE TO VERIFY THE BASIS OF DESIGN BEARING PRESSURE NOTED. DO NOT PLACE CONCRETE PRIOR TO GEOTECHNICAL ENGINEER'S APPROVAL.
FD.2	ASSUMED DESIGN BEARING PRESSURES:-----2000 PSF
FD.3	ALL FOUNDATION BEARING SURFACES SHALL BE INSPECTED AND APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACING CONCRETE TO ENSURE COMPLIANCE WITH PRESSURES NOTED. THE FINAL BEARING ELEVATIONS MAY VARY AS REQUIRED TO PROVIDE PROPER BEARING CAPACITY IN AN APPROVED BEARING STRATUM AS DETERMINED BY THE GEOTECHNICAL ENGINEER.
FD.4	FOOTINGS SHALL BE PLACED THE SAME DAY AS INSPECTION BY THE GEOTECHNICAL ENGINEER UNLESS EXTENDED TIME IS APPROVED BY THE GEOTECHNICAL ENGINEER.
FD.5	FOOTINGS SHALL BE NEATLY EXCAVATED WHERE POSSIBLE WITH SIDES AND TOP EDGES FREE OF LOOSE OR WET MATERIALS. WHERE NEAT EXCAVATION IS NOT POSSIBLE, FOOTING EXCAVATION SHALL BE FILLED WITH CONCRETE TO THE TOP OF FOOTING. THE BOTTOM EXCAVATION SHALL BE CLEAN AND DRY WITH ALL LOOSE MATERIAL REMOVED FOR AN ESSENTIALLY FLAT BEARING SURFACE. WHERE SOFT OR UNSUITABLE BEARING SURFACES ARE ENCOUNTERED, THE AREA SHALL BE UNDERCUT AS REQUIRED AND REPLACED WITH LEAN CONCRETE OR COMPACTED DENSE GRADED CRUSHED STONE AS DIRECTED BY THE GEOTECHNICAL ENGINEER.
FD.6	COMPACTED FILL SHALL MEET THE REQUIREMENTS NOTED IN THE GEOTECHNICAL REPORT. EXCAVATED MATERIAL MAY BE USED AS BACKFILL MATERIAL WITH WRITTEN APPROVAL FROM THE GEOTECHNICAL ENGINEER STATING THAT SUCH MATERIAL IS SUITABLE AS BACKFILL AND INSTRUCTIONS ARE GIVEN FOR PROPER MOISTURE CONTENT AND COMPACTION.
FD.7	PROVIDE 4" OF COMPACTED GRANULAR FILL BENEATH ALL SLABS ON GRADE. PROVIDE 10 MIL VAPOR RETARDER BETWEEN BOTTOM OF SLAB AND TOP OF GRANULAR FILL.
FD.8	FOUNDATIONS SHALL BE CENTERED ABOUT COLUMN LINES, UNLESS NOTED.

CN. CONCRETE

CN.1	CONCRETING OPERATIONS SHALL COMPLY WITH ACI STANDARDS.
CN.2	MINIMUM CONCRETE COMPRESSIVE STRENGTH AT 28 DAYS (PSI), TYPE OF CONCRETE, MAXIMUM W/C (WATER/CEMENTITIOUS MATERIALS RATIO), TOTAL AIR CONTENT, SLUMP AND CONCRETE USE:
	STRENGTH TYPE W/C AIR SLUMP USE
	3000 NORMAL WT. 0.57 4-6% 3" TO 5" UNLESS NOTED
CN.3	REINFORCING BARS: ASTM A615 GRADE 60.
CN.4	WELDED WIRE REINFORCEMENT (WWR): ASTM A185. MINIMUM LAP AND EMBEDMENT TO BE THE GREATER OF ONE CROSS WIRE SPACING PLUS 2" OR 6".
CN.5	REINFORCING STEEL SHOWN IN SECTIONS AND DETAILS IS A SCHEMATIC INDICATION THAT REINFORCING EXISTS. SEE SCHEDULES, SECTION NOTES AND GENERAL NOTES FOR ACTUAL REINFORCING REQUIRED.
CN.6	REINFORCING BAR PLACING ACCESSORIES TO BE INSTALLED IN ACCORDANCE WITH ACI MANUAL OF STANDARD PRACTICE. WHERE CONCRETE IS EXPOSED IN FINISHED BUILDING, PROVIDE ACCESSORIES WITH RUSTPROOF LEGS.
CN.7	DETAIL REINFORCEMENT IN ACCORDANCE WITH ACI 315. REINFORCEMENT SHALL NOT BE WELDED UNLESS NOTED OR APPROVED BY THE STRUCTURAL ENGINEER.
CN.8	SPLICES SHALL BE CLASS "B" TENSION LAP SPLICE, UNLESS NOTED.

CN.9	REINFORCING MARKED "CONTINUOUS" SHALL BE SPLICED WITH CLASS "B" TENSION LAP SPLICE, UNLESS NOTED.
CN.10	CONCRETE COVERAGE OF REINFORCEMENT, UNLESS NOTED: FOOTINGS-----2" TOP & 3" BOTTOM & SIDES

WD. WOOD CONSTRUCTION

WD.1	WOOD FRAMING MEMBERS: VISUALLY GRADED DIMENSIONED #2 SOUTHERN PINE.
WD.2	SILL PLATES, SOLE PLATES AND TOP PLATES SHALL BE OF THE SAME SIZE AS THE STUDS TO WHICH THEY ARE CONNECTED. GRADE SHALL BE AS SPECIFIED ABOVE.
WD.3	ALL PRESSURE TREATED SOUTHERN PINE LUMBER SHALL BE PRESSURE TREATED WITH ALKALINE COPPER QUATERNARY (ACQ) IN ACCORDANCE WITH AMERICAN WOOD PROTECTION ASSOCIATION (AWPA) STANDARD U1, COMMODITY SPECIFICATION A.
A.	USE CATEGORIES:
1.	UC2/INTERIOR DRY - SILL PLATES
B.	ALL FASTENERS, NAILS AND OTHER METAL PRODUCTS USED WITH LUMBER PRESSURE TREATED WITH ACQ SHALL BE HOT-DIP GALVANIZED, STAINLESS STEEL OR AS RECOMMENDED BY THE ACQ MANUFACTURER. PRESSURE TREATED LUMBER SHALL NOT BE IN DIRECT CONTACT WITH ALUMINUM PRODUCTS.
WD.4	ALL MANUFACTURED WOOD FRAMING CONNECTORS TO BE BY SIMPSON STRONG-TIE COMPANY, INC. OR APPROVED EQUAL. ALL CONNECTORS SHALL BE FASTENED TO FRAMING MEMBERS FILLING THE REQUIRED NUMBER OF CONNECTOR HOLES WITH THE TYPE AND SIZE FASTENERS SPECIFIED BY THE MANUFACTURER.
WD.5	ROOF SHEATHING: 3/4" PLYWOOD, APA STRUCTURAL I RATED SHEATHING, EXPOSURE I. PANEL IDENTIFICATION INDEX 48/24. PROVIDE ONE H-CLIP AT ALL ADJOINING PANEL JOINTS MIDWAY BETWEEN SUPPORTS. LONG DIMENSION OF PANEL PERPENDICULAR TO SUPPORTS WITH JOINTS STAGGERED.
WD.6	ROOF SHEATHING NAILING, UNLESS NOTED: 10D HOT-DIPPED GALVANIZED COMMON NAILS AT 6 INCHES AT DIAPHRAGM BOUNDARIES, SHEAR WALLS, DRAG TRUSSES, AND DRAG STRUTS NOTED ON PLAN, 6 INCHES AT PANEL ENDS AND 12 INCHES AT INTERMEDIATE SUPPORTS.
WD.7	SHEAR WALL SHEATHING: 1/2" PLYWOOD, APA STRUCTURAL I RATED SHEATHING, EXPOSURE 1. PANEL IDENTIFICATION INDEX 32/16. LONG DIMENSION OF PANEL PERPENDICULAR TO STUDS (TRUSSES). ALL PLYWOOD EDGES BACKED WITH TWO-INCH NOMINAL OR WIDER FRAMING.
WD.8	SHEAR WALL SHEATHING NAILING, UNLESS NOTED: 10D HOT-DIPPED GALVANIZED COMMON NAILS AT 6 INCHES AT WALL BOUNDARIES, 6 INCHES AT ALL FOUR PANEL EDGES AND 12 INCHES AT INTERMEDIATE MEMBERS.
WD.9	LAMINATED VENEER LUMBER (LVL) BEAMS MANUFACTURED BY WEYERHAEUSER OR LOUISIANA PACIFIC CORPORATION OR APPROVED EQUAL SHALL MEET OR EXCEED THE REQUIREMENTS INDICATED BELOW: BENDING (Fb)-----2850 PSI SHEAR PARALLEL TO GRAIN (Fv)-----285 PSI COMPRESSION PERPENDICULAR TO GRAIN (Fc PER)-----750 PSI MODULUS OF ELASTICITY (E)-----2,000,000 PSI COMPRESSION PARALLEL TO GRAIN (Fc PAR)-----2510 PSI
WD.10	MULTI-PLY ENGINEERED LUMBER BEAMS, UNLESS NOTED OTHERWISE, ARE TO BE FASTENED TOGETHER WITH FASTENMASTER TRUSSLOK FASTENERS OR APPROVED EQUAL WITH A MINIMUM OF TWO ROWS OF FASTENERS AT 12 INCHES (STAGGERED) AND SPACED 3 INCHES FROM THE TOP AND BOTTOM OF BEAMS.
WD.11	REFER TO IBC TABLE 2304.10.1 FOR FASTENING REQUIREMENTS NOT SPECIFICALLY STATED IN DRAWINGS.
WD.12	BOLTS: ASTM A307 GRADE A W/ ASTM A653 HEX NUTS AND WASHERS.
WD.13	NAILS, WIRE BRADS, STAPLES: SHALL CONFORM TO ASTM F1667.
WD.14	POWER DRIVEN FASTENERS: SHALL CONFORM TO NER-272.
WD.15	WOOD SCREWS: SHALL CONFORM TO ASME B18.6.1.

WT. WOOD TRUSSES

WT.1	DESIGN, FABRICATE AND ERECT WOOD TRUSSES IN ACCORDANCE WITH THE "DESIGN SPECIFICATION FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES" OF THE TRUSS PLATE INSTITUTE. ERECTION PLANS, TRUSS CALCULATIONS AND CONNECTION CALCULATIONS, DESIGNED BY THE CONTRACTOR, SHALL BE SUBMITTED FOR THE FILES OF THE STRUCTURAL ENGINEER. CALCULATIONS SHALL BEAR THE SEAL OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE IN WHICH THE PROJECT IS LOCATED.
WT.2	THE WOOD TRUSS SYSTEM ENGINEER SHALL DESIGN THE COMPLETE TRUSS SYSTEM. THE TRUSS SYSTEM IS AN ASSEMBLAGE OF TRUSSES AND TRUSS GIRDERS, TOGETHER WITH ALL BRACING, CONNECTIONS AND OTHER STRUCTURAL ELEMENTS AND ALL SPACING AND LOCALTIONAL CRITERIA THAT, IN COMBINATION, FUNCTION TO SUPPORT THE LOADS APPLICABLE TO THE STRUCTURE.
WT.3	TRUSS MANUFACTURER SHALL DESIGN FOR THE FOLLOWING SUPERIMPOSED LOADS: A. ROOF: 1. TOP CHORD DEAD LOAD -----10 PSF 2. BOTTOM CHORD DEAD LOAD -----10 PSF 3. TOP CHORD LIVE LOAD -----SEE SECTION GN.2.B.2
WT.4	DESIGN ROOF TRUSSES TO RESIST THE WIND UPLIFT LOADING IN ACCORDANCE WITH THE COMPONENTS AND CLADDING TABLE IN THE DRAWINGS.
WT.5	IN ADDITION TO THE ABOVE LOADS, WOOD TRUSSES SHALL BE DESIGNED FOR CONCENTRATED LOADS HUNG FROM OR SUPPORTED ON TRUSSES. REFER TO MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS AND SPECIFICATIONS FOR LOADING INFORMATION AND LOCATION. LOADING AS REQUIRED BY OTHER SUBCONTRACTORS, SUCH AS FIRE PROTECTION, SHALL BE COORDINATED BY THE CONTRACTOR.
WT.6	ALL MANUFACTURED TRUSS HOLD-DOWNS TO BE BY SIMPSON STRONG-TIE COMPANY, INC. OR APPROVED EQUAL. ALL CONNECTORS SHALL BE FASTENED TO FRAMING MEMBERS FILLING THE REQUIRED NUMBER OF CONNECTOR HOLES WITH THE TYPE AND SIZE FASTENERS SPECIFIED BY THE MANUFACTURER.

WT.7 ALL TEMPORARY AND PERMANENT BRACING MEMBERS AND CONNECTIONS REQUIRED FOR WOOD TRUSSES SHALL BE DETAILED ON THE WOOD TRUSS MANUFACTURER'S ERECTION PLANS.

WT.8 TEMPORARY BRACING SHALL NOT IMPOSE ANY FORCE ON THE SUPPORTING STRUCTURE. PERMANENT BRACING FORCES SHALL BE TRANSFERRED TO THE ROOF OR FLOOR DIAPHRAGM BY THE BRACING DESIGN PROVIDED BY THE TRUSS MANUFACTURER.

CF. COLD-FORMED STEEL STRUCTURAL MEMBERS

CF.1	DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS(CFS) AND ACCESSORIES IS THE RESPONSIBILITY OF THE COLD-FORMED STEEL MANUFACTURER. THE CFS MANUFACTURER IS RESPONSIBLE FOR THE DESIGN OF THE CONNECTIONS BETWEEN MEMBERS AND THEIR CONNECTION TO THE BUILDINGS PRIMARY STRUCTURAL FRAME.
CF.2	ANY COLD-FORMED STEEL SIZES NOTED ARE FOR PRELIMINARY PRICING INFORMATION ONLY. THE COMPLETE DESIGN OF COLD-FORMED STEEL FRAMING SYSTEM AND PREPARATION OF ERECTION DRAWINGS ARE BY THE ENGINEER RESPONSIBLE FOR THEIR DESIGN.
CF.3	SUBMIT THE FOLLOWING:
A.	PRODUCT DATA: FOR EACH TYPE OF COLD-FORMED STEEL PRODUCT AND ACCESSORY UTILIZED.
B.	SHOP DRAWINGS: SHOW LAYOUT, SPACINGS, SIZES, THICKNESS, AND TYPES OF COLD-FORMED STEEL; FABRICATIONS; AND FASTENING AND ANCHORAGE DETAILS, INCLUDING MECHANICAL FASTENERS. SHOW REINFORCING CHANNELS, OPENING FRAMING, SUPPLEMENTAL FRAMING, STRAPPING, BRACING, BRIDGING, SPLICES, ACCESSORIES, CONNECTION DETAILS, AND ATTACHMENT TO ADJOINING WORK.
C.	CALCULATIONS: COLD-FORMED STEEL DESIGN CALCULATIONS FOR THE FILES OF THE STRUCTURAL ENGINEER AND ARCHITECT. CALCULATIONS SHALL BEAR THE SEAL OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED.

CF.4	PROVIDE COLD-FORMED STEEL CAPABLE OF WITHSTANDING DESIGN LOADS WITHIN LIMITS AND UNDER CONDITIONS INDICATED.
A.	DESIGN LOADS AS INDICATED IN SECTION GN OF THESE GENERAL NOTES.
B.	DEFLECTION LIMITS: DESIGN FRAMING SYSTEMS TO WITHSTAND DESIGN LOADS WITHOUT DEFLECTIONS GREATER THAN THE FOLLOWING: 1. EXTERIOR LOAD-BEARING WALL FRAMING: HORIZONTAL DEFLECTION OF 1/480 OF THE WALL HEIGHT. 2. INTERIOR LOAD-BEARING WALL FRAMING: HORIZONTAL DEFLECTION OF 1/360 OF THE WALL HEIGHT UNDER A HORIZONTAL LOAD OF 5 LB F/90. FT.
CF.5	VERTICAL STUDS SHALL BE 100% END BEARING.
CF.6	PROVIDE WALL BRACING, CONNECTION DETAILS, AND WINDOW HEADERS AS RECOMMENDED BY THE STUD MANUFACTURER FOR LOAD-BEARING STUDS.
CF.7	VERTICAL STUDS INTERRUPTED BY WALL OPENINGS SHALL BE LOCATED EQUALLY ON EACH SIDE OF THE OPENING. PROVIDE EVEN NUMBER OF FULL HEIGHT STUDS ON EACH SIDE OF OPENING. WELD STUD FLANGES TOGETHER WITH FILLET WELDS AT 6".

SS. STRUCTURAL STEEL

SS.1	FABRICATE AND ERECT ALL STRUCTURAL STEEL IN ACCORDANCE WITH AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".
SS.2	STRUCTURAL STEEL AND STRUCTURAL STEEL CONNECTIONS SHALL CONFORM TO THE FOLLOWING MINIMUM REQUIREMENTS UNLESS NOTED OTHERWISE: W AND WT SHAPES ASTM A992
	STIFFENER PLATES, CAP PLATES, CONNECTION PLATES, AND ANGLES ASTM A36
	BASE PLATES ASTM A572, GRADE 50
	HOLLOW STRUCTURAL SECTIONS ASTM A500, GRADE C
	WELDED CONNECTIONS E70XX ELECTRODES, MINIMUM SIZE FILLET WELD 3/16"
	BOLTS ASTM A325 OR A490
	NUTS ASTM A563
	WASHERS ASTM F436
SS.3	WHERE NO CAMBER IS INDICATED, BEAMS SHOULD BE ERECTED WITH NATURAL CAMBER ORIENTED UPWARD.
SS.4	HSS MEMBERS SHALL HAVE A 1/4" CLOSURE PLATE.
SS.5	STRUCTURAL STEEL MEMBERS SHALL NOT BE CUT, SPLICED, OR MODIFIED IN THE FIELD UNLESS NOTED ON THE STRUCTURAL DRAWINGS OR APPROVED BY THE STRUCTURAL ENGINEER.



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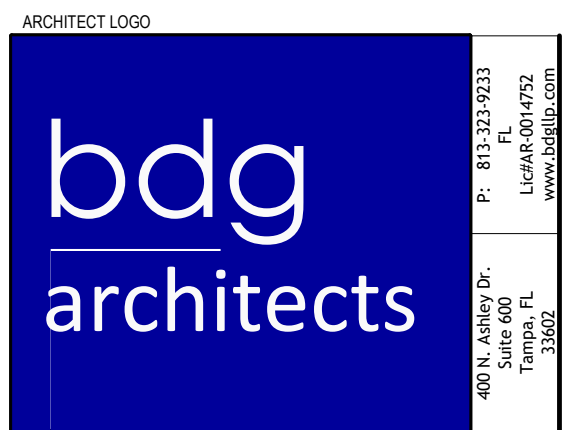
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GENERAL NOTES