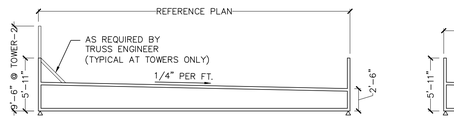
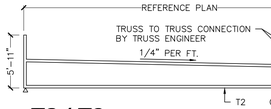


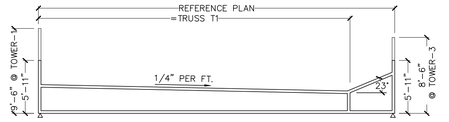
T1 / GT1

SCALE: 1/8" = 1'-0"



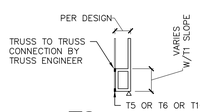
T2 / T3

SCALE: 1/8" = 1'-0"



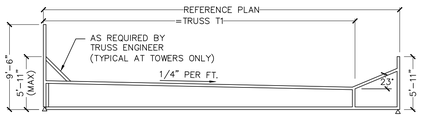
T5

SCALE: 1/8" = 1'-0"



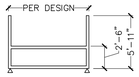
T8

SCALE: 1/8" = 1'-0"



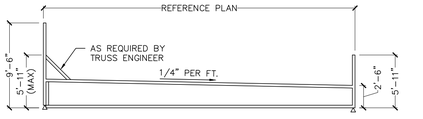
T6

SCALE: 1/8" = 1'-0"



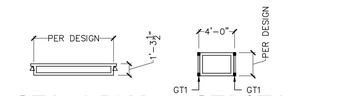
GT3

SCALE: 1/8" = 1'-0"



T7

SCALE: 1/8" = 1'-0"

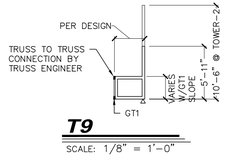


GT4 - 3 PLY

SCALE: 1/8" = 1'-0"

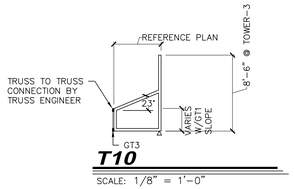
GT2 / T4

SCALE: 1/8" = 1'-0"



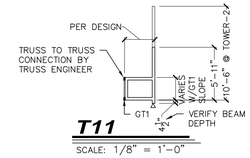
T9

SCALE: 1/8" = 1'-0"



T10

SCALE: 1/8" = 1'-0"



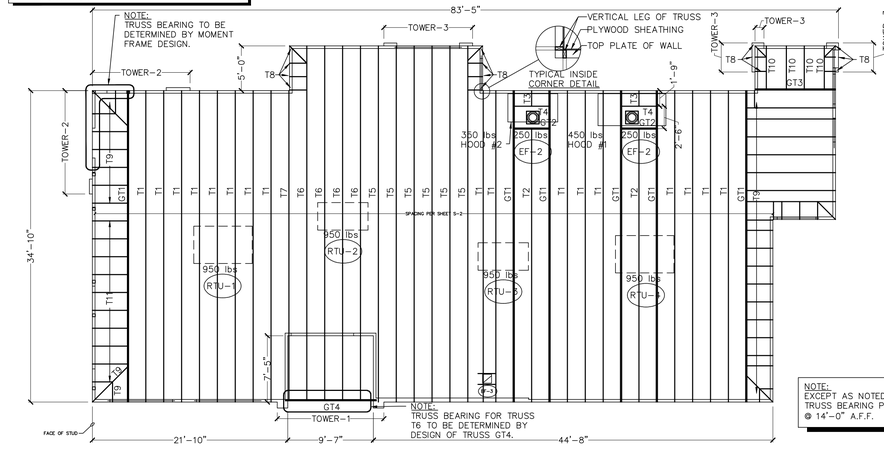
T11

SCALE: 1/8" = 1'-0"

- TRUSS PROFILE PROVIDE FOR DESIGN INTENT ONLY. FIELD MEASUREMENTS REQUIRED FOR SITE SPECIFIC PROJECT.
- TRUSS DESIGN/MEMBER SIZES TO BE DETERMINED BY THE TRUSS ENGINEER BASED ON LOCAL DESIGN CRITERIA ON S-3.1/2.
- REFER TO GENERAL NOTES FOR ADDITIONAL INFORMATION

TRUSS PROFILES

SCALE: 1/8" = 1'-0"



TRUSS LAYOUT PLAN

SCALE: 1/8" = 1'-0"

NOTE:
EXCEPT AS NOTED, ALL TRUSS BEARING POINTS @ 14'-0" A.F.F.

WOOD FRAMING NOTES

1. For wood connections not specifically noted or detailed, follow the requirements of IBC 2006 Table 2304.9.1 or ESR 1539.
2. All nails are common nails unless noted otherwise. All nails shall be carefully driven and not overdriven. Submit all proposed fasteners for approval prior to construction. Installation of all fasteners shall meet the requirements of NDS and ISANTA guidelines, including those in ESR 1539, and section 2303.6 of the IBC.
3. Refer to the Wall Schedules and/or Framing Plans for size, spacing, and species of wall studs and plates. If not shown otherwise, studs and plates are to be #1 or #2 Spruce-Pine-Fir (SPF) with stud spacing 16" o/c maximum. If nor shown otherwise, bearing wall headers are to be #2 Southern Pine (SYP).
4. At the contractor's discretion, studs in non-load bearing interior wall may be premium stud grade at 16" o.c. on all levels.
5. Fasten double (DBL) studs together with 0.131" x 3" nails at 6" o/c U.N.O. For more than two studs fasten in the same way nailing as each stud is added.
6. All headers in non-bearing interior walls are to be 2x4 #2 SPF for openings up to 4'-0" & 2x6 #2 SPF for openings over 4'-0". Use 2 members in 2x4 walls, and 3 members in 2X6 walls. All headers in non-load bearing walls to have (1) jack stud at each end.
7. Refer to the Shear Wall Schedule for sheathing, nailing, strap ties, hold downs, etc. required for wood-panel-sheathed and gypsum-wallboard-sheathed shear walls.
8. Use double top plates on all walls including non-load bearing walls with all splices, corners & intersections lapped.
9. Unless otherwise noted on plan or detail, anchor wall plates to foundations and/or supporting structure using Simpson Strong Tie Titen HD Heavy Duty Screw Anchors, 1/2" diameter with minimum 5" embedment. Space anchors at 48" o.c. for load bearing and non-load bearing walls. Reduce spacing to 24" o.c. for all shear walls.
10. Coordinate final floor and roof framing including joist or truss layout & truss member configuration with Mechanical, Electrical, & Plumbing drawings (MEP). Obtain additional MEP information as needed for complete coordination. Keep all mechanical chases free of framing. Do not locate joists or trusses at parallel plumbing walls.
11. Always bear floor and roof joists or trusses on available interior and exterior bearing walls. Do not clear span framing disregarding an available bearing wall where such a bearing wall is identified.
12. Where floor trusses are used, use a minimum of (2)4x2 vertical members in floor trusses at all bearings unless noted otherwise. One of these verticals may be under a ribbon board at the end of the truss where ribbon boards are allowed. Do not allow for, nor use ribbon boards at the ends of trusses where solid, continuous full-height blocking or continuous wood sheathed knee walls are indicated to be used. Where ribbon boards are used with floor trusses they are to be 2x6 minimum.
13. Design roof joists or trusses to support the weight of snow drifting where it applies as well as roof top mechanical units, exhaust fans, access hatches, etc. Confirm weights & locations before final design. Show the loads for these units/fixtures on the sealed drawings. The Contractor shall ensure the units are installed at their design locations.
14. Where framing supported by a joist or truss can cause uplift on that joist or truss (such as at cantilevered balcony framing) the designer shall run a load case that maximizes the uplift load in combination with no live loads on the joist or truss supporting the uplift.
15. All exposed framing to be pressure preservative treated wood (PPT), reference specs. All PPT wood to be kiln dried after treatment (KDAT). Hardware used with PPT wood to be HDG or S.S.
16. All hardware to be Simpson Strong Tie, U.N.O. Where hardware is not specifically designated, submit proposed hardware for approval. Where more than one type of fastener or fastener pattern is allowed by the hardware manufacturer, hardware fasteners are to be of the type, size, and quantity to maximize the load capacity of the hardware in the specific application shown on these plans U.N.O.
17. Reference the Architectural Plans for layout of all walls, openings, wall types, etc. Verify all dimensions prior to design of wall panels & immediately notify the Architect and Engineer of any discrepancies.
18. Where a Specialty Structural Engineer (SSE) designs floor and/or roof framing (such as trusses or I-joists) at the bearing of all girders and beams, the floor and/or roof designer shall provide the Wall Panel Designer the loads/reactions and locations of all girder and beams. The Wall Panel Designer shall specify and the Wall Panel Manufacturer shall install sufficient columns/studs to support all such loads from the girder's and beam's bearings down to the supporting foundation or podium framing. The Contractor shall ensure the presence of such columns/studs. Similarly, where walls are field-framed, the Framing Contractor shall install the columns/studs for support of girders and beams. As a minimum, the number of studs shown on these plans shall be used with a minimum of (2) 2x6 or (3) 2x4 studs.
19. At bearing walls, blocking must be added in the floor system to create continuity of all shear wall chord studs, posts and columns, jack/king studs at headers, etc. Such blocking shall be part of the sheathed shear blocking panels, or knee walls where they are used.
20. Where decks or balconies are wood framed, if the railing relies on its connection to the wood balcony beams or rim joists for stability and load resistance then the railing designer shall check torsion and other effects on the edge beams or joists and their connections.

DIMENSIONS OF COMMON NAILS

Pennyweight	Min. Length (inches)	Shank Diameter (inches)
6d	2	0.113
8d	2.5	0.131
10d	3	0.148
16d	3.5	0.162
20d	4	0.192

Notes:
1. Nails called out in plan, section, detail, or schedule are always common nails. nail diameter is per ESR-1539, NDS, and the table above except that nail length will always be .5" minimum when nailing 2X framing members together and 3.5" when nailing LVL's.
2. For connections not shown, refer to IBC table 2304.91 for minimum fastening requirements.
3. For fastening of multiple LVL plies, follow the LVL manufacturer's requirements.
4. For fastening of Simpson Strong-Tie and other hardware, follow the hardware manufacturer's requirements. Fill all fastener holes with the required fasteners unless otherwise noted.

PREFABRICATED, PLATE-CONNECTED WOOD TRUSSES

1. Wood trusses shall be designed by the Truss Manufacturer to support the following loads:

   A) Roof Truss, Typ. (See load diagram below for additional loading at corridors):
   Top Chord Loading:
   Live Load: 20 psf
   Snow Load: 21 psf + Drift Loads per plan
   Dead Load: 15 psf

   Bottom Chord Loading:
   Dead Load: 10 psf

   Wind Loading: Per the 2015 IBC.

   Trusses shall meet the following deflection criteria, unless otherwise noted.
   Maximum live load deflection shall not exceed:

   A) Roofs with suspended ceilings: L/360
   B) Roofs with no ceilings: L/240

2. Wood trusses shall be designed by the Truss Manufacturer in accordance with the applicable provisions of the latest edition of the National Design Specification of the American Forest & Paper Association, The Design Specification for Metal Plate Connected Wood Trusses of the Truss Plate Institute (TPI), Wood Structural Building Components Association (SBCA) and the Mississippi Uniform Construction Code (UCC).
3. Wood trusses shall be fabricated by a Truss Manufacturer who maintains written procedural and quality control manuals and engages in periodic auditing of fabrication practices and inspections as required by ANSI/TPI 1, Chapter 3. In addition, the Truss Manufacturer shall engage in periodic, unannounced auditing by an approved third-party inspection agency such as TPI for review. If the Truss Manufacturer cannot fulfill these requirements, the SER may, at his discretion, require or accept from the Truss Manufacturer, a letter sealed by a Professional Engineer (SSE) registered in the State of Mississippi verifying his inspection of the manufactured wood trusses supplied for this project. The SSE shall verify that the trusses are in compliance with the quality standards of ANSI/TPI 1, Chapter 3. The SSE shall have significant prior experience in the inspection of metal plate-connected wood trusses.
4. Wood materials shall be Southern Pine, SPF, or Douglas Fir-Larch and shall be kiln-dried and used at 19% maximum moisture content. Provide Grade No. 2 or better as required to satisfy stress requirements.
5. Connector plates shall be not less than 0.036 inches (20 gauge) in coated thickness, shall meet or exceed ASTM Grade A or higher, and shall be hot-dip galvanized according to ASTM A653 (coating G60). Minimum yield stress shall be 33,000 psi.
6. Trusses shall be fabricated in a properly equipped manufacturing facility of a permanent nature. Trusses shall be manufactured by experienced workmen, using precision cutting, jigging and pressing equipment under the requirements of quality control as indicated in ANSI/TPI 1, Chapter 3.
7. Secondary bending stresses in top and bottom chords due to dead, live, and wind loads shall be considered in the design. Load duration factors shall be per the "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION".
8. All girder trusses supporting other trusses or 2x framing members shall be a MINIMUM of (2) plies, unless otherwise approved. Refer to the Manufacturer's Truss Design Drawing for girder ply-to-ply connection requirements. Attach framing members or loads only after all girder plies are in place and properly fastened together, and the girder truss is properly braced to prevent lateral displacement. Refer to BCSI-B9 "Multi-Ply Girders" as published by the SBCA and TPI for additional information.
9. Truss-to-girder connection information shall be shown on the Manufacturer's Truss Design Drawing of the carried truss or girder truss, or the Manufacturer's Truss Placement Drawing. Unless otherwise approved, all truss/joist hangers, straps, ties, etc. shall be as manufactured by the Simpson Strong Tie Company.
10. Unless otherwise shown or noted, ALL truss bearings shall be anchored using a mechanical fastener. As a minimum, provide H2.5A Wind/Seismic anchors as manufactured by the Simpson Strong Tie Company.
11. Wood trusses shall be erected in accordance with the Truss Manufacturer's requirements. This work shall be performed by a qualified and experienced contractor. Truss erection by an inexperienced or non-qualified contractor can result in construction collapse and/or serious injury and damage.
12. Where long span trusses (greater than 60' span) are used, refer to BCSI (especially BCSI-B1 and BCSI-B2) special recommendations and requirements for the installation and temporary restraint/bracing of long span trusses. Follow all BCSI recommendations and requirements for the installation and temporary restraint/bracing of long span trusses including consulting a registered Professional Engineer familiar with the installation and temporary restraint/bracing of long span trusses.
13. The Contractor shall provide all temporary and permanent bracing/restraints as required for safe erection and performance of the trusses. The guidelines set forth by the following joint publications of the Truss Plate Institute (TPI) and the Structural Building Components Association (SBCA) shall be adhered to unless otherwise shown in the Contract Documents.
    - BCSI-B1 GUIDE FOR HANDLING, INSTALLING AND BRACING OF METAL PLATE CONNECTED WOOD TRUSSES.
    - BCSI-B2 TRUSS INSTALLATION AND TEMPORARY BRACING.
    - BCSI-B3 WEB MEMBER PERMANENT BRACING/WEB REINFORCEMENT
    - BCSI-B4 CONSTRUCTION LOADING
    - BCSI-B5 TRUSS DAMAGE, JOBSITE MODIFICATIONS AND INSTALLATION ERRORS
    - BCSI-B6 GABLE END FRAME BRACING
    - BCSI-B7 TEMPORARY AND PERMANENT BRACING FOR PARALLEL CHORD TRUSSES
    - BCSI-B8 TOE-NAILING FOR UPLIFT REACTIONS
    - BCSI-B9 MULTI-PLY GIRDERS
    - BCSI-B10 POST FRAME TRUSS INSTALLATION AND BRACING
    - BCSI-B11 FALL PROTECTION AND WOOD TRUSSES
14. Unless otherwise shown or noted, permanent bracing/restraint shall consist of 2x4 minimum stress-graded members spanning a minimum of four trusses and nailed at each intersection with a minimum of two 16d nails. Lap continuous bracing/restraint a minimum of 2'-0" (2 trusses).
15. Refer to the Manufacturer's Truss Design Drawing for web members requiring web member permanent web restraint or reinforcement. Continuous lateral restraints must ALWAYS be diagonally braced for rigidity.
16. Wherever possible, the temporary erection bracing as described in BCSI-B2 shall be left in place to function as permanent bracing.
17. "I" or "L" reinforcement of the type described in BCSI-B3 shall be used as the means of resisting web member buckling forces in girder trusses and small-quantity trusses. Girder trusses shall be those trusses supporting other trusses, beams, or framing, or trusses more than 24" o.c. Small quantity trusses shall be those where because of the small quantity of similar trusses with aligned web members requiring bracing/restraint (such as parallel step-down trusses), the installation of continuous lateral restraints and the diagonal bracing for the continuous lateral braces is impractical using the methods of BCSI. Such "I" or "L" reinforcement shall be designed by the Truss Design Engineer and shown on the sealed Truss Design Drawings.
18. Design all wood truss bearings using the compression perpendicular-to-grain value of the truss lumber OR the wall plate lumber/nailer, WHICHEVER IS LESS. The bearing length or area shown on the sealed truss drawings shall be based on this lesser value. Use additional truss plies or truss bearing enhancement devices to achieve the required bearing area. If the bearing enhancement device has a valid ESR report, it may be shown on the Truss Placement Plan instead of being shown on the sealed Truss Drawings.
19. Trusses which are too tall for delivery to the jobsite in one piece may be manufactured in two or more sections and "piggybacked" at the jobsite. The Contractor MUST install all temporary and permanent bracing for the lower supporting trusses as shown on the Manufacturer's Truss Design Drawing and/or the Contract Documents BEFORE installing the cap trusses. The connection between the cap and base truss shall be shown on the sealed Truss Design Drawing. Provide, as a minimum, 4x2 sleepers laid flat on the top chord of the supporting trusses, spaced at 24" o.c. maximum and nailed to the top chord with a minimum of two 16d nails (see Truss Design Drawing for any other or additional requirement). Provide 4' o.c. (brace to brace) diagonal bracing at 45 degrees to brace the sleepers. Install diagonal braces on the bottom side of the top chord in the cap area, unless otherwise specified by the Truss Design Engineer.

PREFABRICATED, PLATE-CONNECTED WOOD TRUSSES (CONT.')

20. Truss members and components shall not be cut, notched, drilled nor otherwise altered in any way without the written approval of the Truss Design Engineer (see Specialty Structural Engineer notes).
21. Where trusses and/or wood joists and rafters are to be supported by steel beams, provide continuous 2x nailer(s) field-bolted to the top of the steel beam with ½" diameter carriage bolts spaced a maximum of 24" on center and staggered each side of the beam web. Width of nailer to equal width of the beam flange + ½"/- ¼", unless otherwise noted. Carriage bolts may be over-tightened to compress the rounded head in the nailer to facilitate installation of continuous band/rim joists, rafters, trusses, etc..
22. Coordinate final truss layout and truss web member configuration with mechanical, electrical and plumbing trades. Keep all mechanical chases free of trusses. Align floor truss web layout, including open Verendeel panels for all parallel trusses with similar spans to provide maximum flexibility of MEP installation. Do not locate trusses at parallel plumbing walls.
23. Submit complete shop drawings for all wood trusses showing member sizes, species, grade, moisture content, span, camber, dimensions, number of plies and truss ply-to-ply connections, reactions and bearing requirements, chord pitch, restraint/bracing requirements, loadings including any girder loads, field assembly of multi-part trusses, and required cap truss to base truss connection. All required information must be complete on the individual truss drawing. No reference to another drawing sheet, note external to that individual truss drawing, or detail not shown on that individual truss drawing may be used to convey any of this required information. Shop Drawings shall be submitted to the SER and shall bear the seal of a Professional Engineer (SSE) registered in the State of Mississippi.

STRUCTURAL WOOD PANEL/ SHEATHING NOTES

1. All plywood and Oriented Strand Board (OSB) construction shall be in accordance with the American Plywood Association (APA) Specifications and DOC PS1 or PS2.
2. All roof sheathing shall be 5/8" nominal, APA-rated Exterior sheathing. Fasten sheathing with 8d common nails spaced 6" o.c. at supported edges and 6" o.c. at intermediate supports.
3. All structural wood panel wall sheathing shall be 1/2" nominal, APA-rated PS2 sheathing unless otherwise noted. Fasten wall sheathing with 8d common (0.131" x 2½") nails spaced 6" o.c. at supported edges and 12" o.c. at intermediate supports, unless otherwise noted in the Shear Wall Schedule.
5. Provide 2x blocking with specified edge nailing at unsupported panel edges as follows:
   A) Roofs - Not required unless indicated on the plans, notes, or details.
   B) Walls - Required at all wood panel joints, unless noted otherwise.
6. Unless otherwise noted or shown, install roof sheathing with the long dimension of the panel across supports and with the panel continuous over two or more spans. Stagger panel end joints. Allow ⅛" spacing at panel ends and edges unless otherwise recommended by the sheathing manufacturer.
7. Wood structural panels used in shear walls shall be 4' x 8' minimum. These panels may only be cut at wall or wall opening boundaries. All panel edges shall fall on framing members. Block all horizontal joints and fasten with edge nailing.
8. In all wood-framed roof, ceiling, and wall areas where wood sheathing and/or gypsum wall board sheathing is applied, attach the sheathing to all wood framing members regardless of the closeness of their spacing. Where gypsum wall board sheathing is applied over resilient channels, attach the resilient channels to all framing members.
9. Installation of gypsum panel products must follow the requirements of the Gypsum Association. The "Floating Interior Angles" method as described in GA-216 shall be used to avoid negative effects from potential truss uplift, wood shrinkage, and other causes of framing movement. An extensive discussion of this issue can be found in the TTB "Partition Separation Prevention and Solutions" from the Structural Building Components Association. The contractor shall familiarize himself with the content of these documents before beginning the installation of gypsum panel products.
10. The requirements shown on the structural drawings for sheathing are the minimum requirements for the structural needs of the structure. They do not account for all possible quality, aesthetic, and other considerations. The contractor is expected to be familiar with APA's construction guidelines and other common construction practices necessary to avoid quality and aesthetic issues. The use of panel edge gaps to avoid panel buckling is an example. Another is the allowance of the application of wood sheathing to walls with the face grain vertical, which can lead to greater buckling possibilities. The contractor will need to consider stud size, stud spacing, and sheathing thickness in these situations.

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ROC-60 TALL 20/20 IMAGE: NOVEMBER 2018 DESIGN RELEASE

STRUCTURAL NOTES & SCHEDULES (CONT.)

S-3.2